

Influence of Infill Panels and their Distribution on Seismic Behavior of Existing Reinforced Concrete Buildings

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Abstract: Infills can highly influence the seismic response of a Reinforced Concrete (RC) building: generally speaking, their presence leads to an increase in global stiffness and strength, but their brittle behavior can result in an increase of displacement demand if a certain threshold of seismic intensity is overcome. Moreover, presence of infills often leads to a change in the collapse mechanism compared with the bare structure, leading, for instance, to column-sway storey mechanisms characterized by a detrimental localization of inelastic displacement demand. In this paper, a numerical investigation of the influence of infills on the seismic behavior of a case-study existing gravity load designed RC building is carried out. Different infill configurations are considered (Bare, Uniformly infilled and Soft-storey infilled). Seismic capacity assessment is carried out by means of Static Push-Over analyses, within the N2 spectral assessment framework. A sensitivity analysis is carried out, thus evaluating the influence of main material and model parameters on seismic response at different Limit States, namely Damage Limitation and Near Collapse, mainly due to the change in parameters as effective period of vibration, base shear and displacement capacity.

Keywords: Existing, infills, infill distribution, reinforced concrete, seismic capacity, sensitivity analysis.

INTRODUCTION

A growing attention is addressed to the influence of infills on the seismic behavior of Reinforced Concrete (RC) buildings. Their presence leads to an increase in lateral stiffness, thus modifying the dynamic properties of the structure, resulting in a lower period of vibration [1]. The local interaction between the panels and the adjacent structural elements can lead to brittle failure mechanisms [2], also of a kind that may not be considered in an usual seismic design or assessment: for instance, sliding shear failure may take place at the interface between column element and joint panel [3]. During last decades, the importance of this issue was widely recognized by earthquake engineering researchers, also based on the observation of damage to RC buildings with infills after severe earthquakes (e.g., Kocaeli 1999 [4]), leading to first full-scale experimental tests on infilled RC frames [5] and code prescriptions about the consideration of infills in seismic design [6]. As a result, from the second half of 1990s on, several valuable numerical efforts have been made to investigate the seismic behavior of RC frames with infills through nonlinear analyses. Fardis and Panagiotakos [7] and Kappos *et al.*, [8] evaluated the influence of infills on the seismic response of RC frames designed for seismic loads according to contemporary earthquake engineering principles [6]. Further studies [9, 10] focused the attention on different issues of seismic behavior of infilled frames such as the localization of displacement demand and the increase in stiffness and strength, also based on experimental

results [5, 11]. Fundamental studies were carried out by Dolšek and Fajfar: the authors analyzed the influence of infills on seismic demand depending on their mechanical characteristics and the design typology of the RC structure [12] and modeling issues of infilled RC frames [13]. In [14] the authors proposed a R- μ -T relationship accounting for the typical degrading force-displacement response of an infilled RC frame, in order to carry out the seismic assessment of infilled RC frames through the N2 method [15]. Such relationship was applied to the seismic assessment of two case-study structures in [16]. The same procedure was applied to a case study structure in different infill configurations in [17] and [18], leading to a simplified estimation of failure probability at different Limit States, taking into account the influence of the uncertainty in capacity and response of infill elements. The influence of uncertainty on seismic capacity of infilled RC frames was analyzed in [19] through the construction of fragility curves and in [20] by means of a sensitivity analysis.

In this paper, a numerical investigation on the influence of infills on the seismic behavior of a case-study Gravity Load Designed (GLD) building is carried out by means of Static Push-Over (SPO) analyses, within the N2 spectral assessment framework. Different infill configurations are considered (Bare, Uniformly infilled and Soft-storey infilled), and a sensitivity analysis is carried out, thus evaluating the influence of main material and capacity parameters on seismic response for different Limit States.

CASE STUDY STRUCTURE: NUMERICAL MODELING AND ANALYSIS METHODOLOGY

The case study structure is a GLD building, defined by means of a simulated design procedure according to code prescriptions and design practices in force in Italy between

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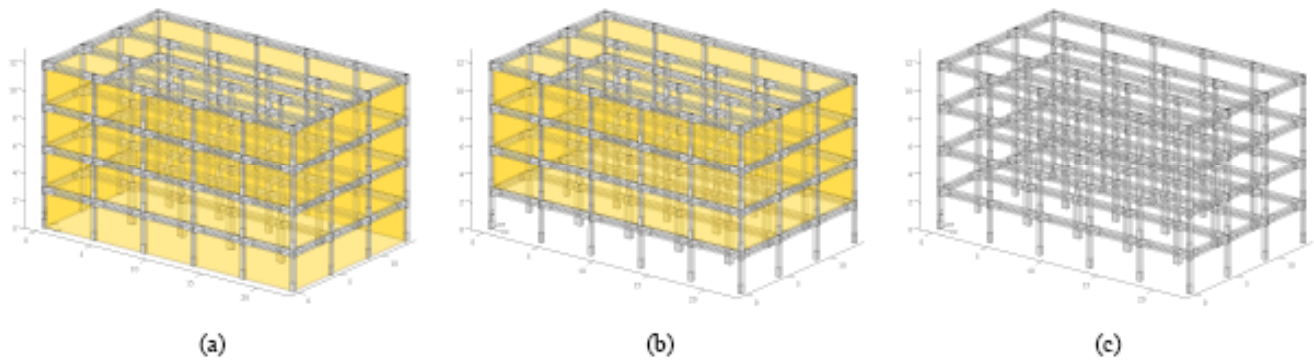


Fig. (1). Uniformly infilled (a), Pilotis (b) and Bare (c) frames

1950s and 1970s [21, 22]. The building is symmetric in plan, both in longitudinal (X) and in transverse (Y) direction. It is a four-storey building, with five bays in longitudinal direction and three bays in transverse direction. Interstorey height is equal to 3.0 m, bay length is equal to 4.5 m. The structural configuration follows the parallel plane frames system: gravity loads from slabs are carried only by frames in longitudinal direction. Beams in transverse direction are present only in the external frames. Slab way is always parallel to the transverse direction. Element dimensions are calculated according to the allowable stresses method; the design value for maximum concrete compressive stress is assumed equal to 5.0 and 7.5 MPa for axial load and axial load combined with bending, respectively. Column dimensions are calculated according only to the axial load, beam dimensions and reinforcement are determined from bending due to loads from slabs. Reinforcement in columns corresponds to the minimum amount of 0.8% of the section area, as prescribed by code [21]. Reinforcing bars are smooth.

Three hypotheses are made for infills:

Case 1: infill panels are uniformly distributed along the height (Uniformly infilled frame, see Fig. (1a)).

Case 2: first storey is bare and upper storeys are infilled (Pilotis frame, see Fig. (1b)).

Case 3: no infill panel is present (Bare frame, see Fig. (1c)).

Infill panels, if present, are uniformly distributed in all the external frames of the building. Panel thickness is equal to 20cm. Presence of openings is not taken into account.

Nonlinear response of RC elements is modelled by means of a lumped plasticity approach: beams and columns are represented by elastic elements with rotational hinges at the ends. A three-linear envelope is used, characteristic points are cracking, yielding and ultimate. Section moment and curvature at cracking and yielding are calculated on a fiber section, for an axial load value corresponding to gravity loads. The behavior is assumed linear elastic up to cracking and perfectly-plastic after yielding. Rotations at yielding and ultimate are evaluated through the formulations given in [23]. No reduction of ultimate rotation for the lack of seismic detailing is applied, due to the presence of smooth reinforcement [24].

Infill panels are modelled by means of equivalent struts. The adopted model for the envelope curve of the force-displacement relationship is the model proposed by in [25, 26]. The ratio between post-capping degrading stiffness and elastic stiffness (parameter α) is assumed equal to 0.03. The ratio between residual strength and maximum strength (parameter β) is assumed equal to 0.01.

Nonlinear SPO analyses are performed on the case study building both in X and Y direction. The assumed lateral load pattern is proportional to the displacement shape of the first mode. Lateral response is evaluated in terms of base shear-top displacement relationship. Structural modelling, numerical analyses and post-processing of damage data, including the 3D graphic visualization of the deformed shape, are performed through the “PBEE toolbox” software [27], combining MATLAB® with OpenSees [28], modified in order to include also infill elements [20, 29]. Each SPO analysis run took about 180 or 90 seconds on a standard Intel® Core™ 2 Duo processor, if infills were present or not in the numerical model, respectively.

When the lateral response is characterized by a strength degradation due to infill failure, a multi-linearization of the pushover curve is carried out by applying the equal energy rule respectively between the initial point and the maximum resistance point, between the maximum resistance point and the point corresponding to the last infill failure, between the point corresponding to the last infill failure and the point corresponding to the first RC element conventional collapse. When the lateral response is not characterized by a strength degradation (because infill elements are not present or not involved in the collapse mechanism) an elasto-plastic bi-linearization is carried out by applying the equal energy rule between the initial point and the maximum resistance point. Moreover, the procedure proposed in [16] to improve the accuracy of the displacement demand assessment in the case of low seismic demand is applied, by approximating the first part of the pushover curve by a bilinear curve rather than a linear one and applying specific R- μ -T relationships in this range of behavior, as proposed by the authors.

Two limit states are defined: Damage Limitation (DL), corresponding to the displacement when the last infill in a storey reaches its maximum resistance thus starting to degrade [17] or when the first yielding in RC members oc-

curs, and Near Collapse (NC), corresponding to the first conventional collapse in RC members.

IN2 curves [30] for the equivalent SDOF systems are obtained by assuming as Intensity Measure both the elastic spectral acceleration at the period of the equivalent SDOF system ($S_{ae}(T_{eff})$) and the Peak Ground Acceleration (PGA). Values of these seismic intensity parameters corresponding to characteristic values of displacement (ductility) demand (including the considered Limit States) are calculated, based on the R - μ - T relationships given in [14] or in [31] for degrading or non-degrading response, respectively.

The R - μ - T proposed by Dolšek and Fajfar [14] is aimed at evaluating the inelastic displacement demand starting from elastic demand spectra based on SDOF systems characterized by the typical idealized force-displacement envelope of an infilled RC frame see Fig. (2).

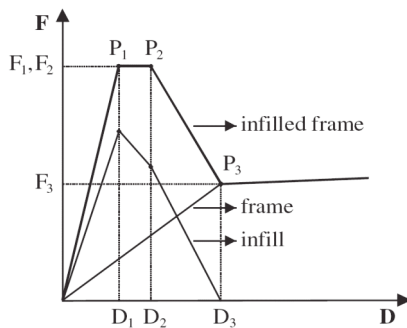


Fig. (2). Force-displacement envelope of the SDOF system [14].

The first, equivalent elastic part represented both the initial elastic behavior and the behavior after cracking has occurred in both the frame and the infills. The second part, corresponding to the horizontal branch, represented yielding. The third part represented the strength degradation of the infills. Then, the horizontal branch represented the stage when infills are failed and only the RC frame resists the horizontal actions. A parametric study was carried out executing nonlinear dynamic analyses on the considered SDOF system with three sets of 7 accelerograms, varying different parameters:

- T/T_C : ratio between the period of the SDOF system and the corner period of the ground motion;
- r_u : ratio between the residual strength and the maximum strength (F_3/F_1 , see Fig. (2));
- μ_s : ductility at the beginning of the degradation (D_2/D_1);

- μ_u : ductility at the end of the degradation (D_3/D_1).

Based on obtained results, an increase in ductility demand was observed with decreasing r_u . Moreover, it was observed that μ_u had a negligible influence on ductility demand. Hence, this parameter was not included in the proposed relationship. It can also be observed that, from a qualitative standpoint, based on this relationship, the sudden increase in displacement demand observed when a soft-storey occurs in uniformly infilled structures for a seismic demand exceeding a certain threshold (e.g., [12]) may be explained not only by the localization of the displacement demand, but also by the typical brittle behavior shown by the structural response when this is controlled by the response of a storey where the infills fail; such a strength drop is represented through the parameter r_u . Based on such relationship, seismic assessment of infilled RC buildings can be included within the spectral framework of the N2 method [15], through SPO analyses.

Elastic spectra are the Uniform Hazard Newmark-Hall demand spectra adopted in Italian code [32] – provided by [33] – for a high seismic city in Southern Italy (Avellino, Lon.: 14.793 Lat.: 40.915). Soil type A (stiff soil) and 1st topographic category are assumed (no amplification for stratigraphic or topographic effects). It is worth noting that a double iterative procedure is required to evaluate $S_{ae}(T_{eff})$ and PGA from the characteristic parameters of equivalent SDOF system – namely the ductility at the point of interest (μ), the period (T_{eff}) and for degrading systems also the ductility at the beginning of the degradation (μ_s) and the ratio between the residual strength and the maximum strength (r_u) – for the following reasons:

- The spectral shape depends on some parameters, such as the corner period (T_C) and the ratio between the spectral acceleration on the constant branch and the PGA (F_0), which are not constant with the seismic intensity (i.e., with the return period), hence also the ratio between $S_{ae}(T_{eff})$ and PGA changes with the seismic intensity;
- Some characteristic parameters of the elastic spectrum, such as T_C , are input parameters for the R - μ - T relationship, but also depends on the results obtained from the R - μ - T relationship since they depends on the seismic intensity.

Due to the fact that the ratio between $S_{ae}(T_{eff})$ and PGA is not constant, the IN2 curves in terms of $S_{ae}(T_{eff})$ or in terms of PGA may have different shapes.

Table 1. Summary of median and CoV values for the selected Random Variables.

R.V.	Median Value	Reference	Distribution	CoV	Reference
f_c	25.0 MPa	[35]	Lognormal	0.310	[35]
f_y	369.7 MPa	[36]	Lognormal	0.080	[36]
G_w	1240 MPa	[26]	Lognormal	0.300	[19, 37, 38]
θ_y	1.015*calculated	[23] (Eq. 2.20a, Table 2.4)	Lognormal	0.331	[23] (Table 2.4)
θ_u	0.995*calculated	[23] (Eq. 3.27a, Table 3.2)	Lognormal	0.409	[23] (Table 3.2)

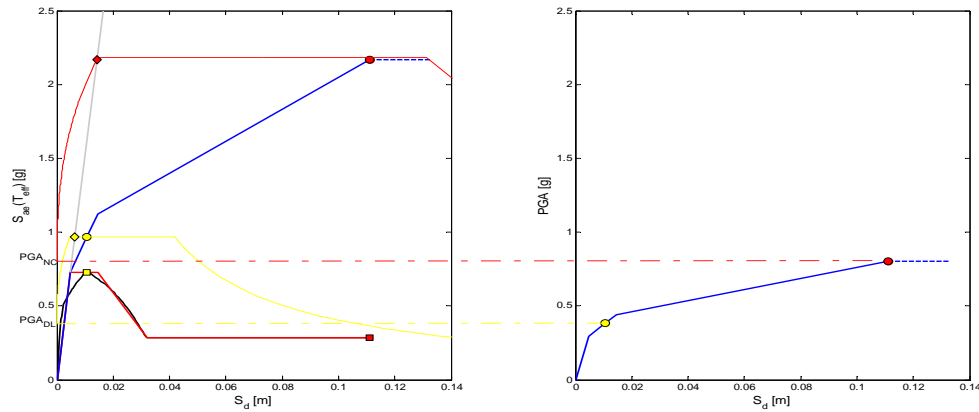


Fig. (3). Schematic construction of an IN2 curve in terms of $S_{ab}(T_{eff})$ (a) and PGA (b) for Model #1 – Uniformly infilled frame – X direction (elastic demand spectra at DL and NC are reported as solid yellow and red curves, respectively; diamonds and squares correspond to elastic and inelastic acceleration-displacement demand points, respectively).

Uniformly infilled frame – X direction

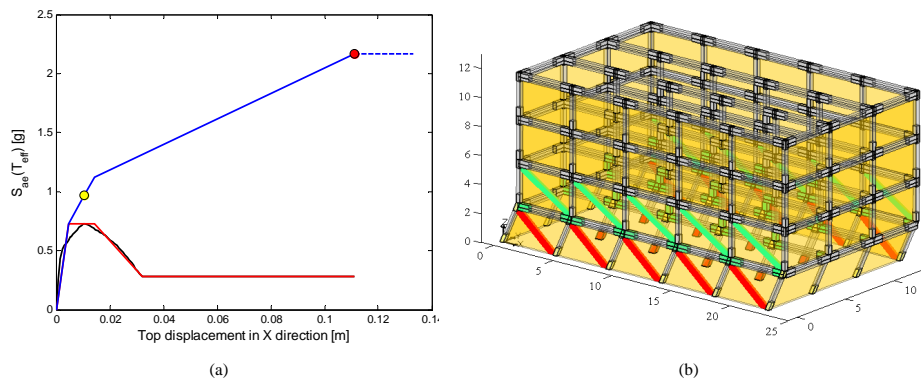


Fig. (4). Obtained (black) and idealized (red) pushover curves and IN2 (blue) curve (a); deformed shape and element damage at NC (b) (Model #1 – Uniformly Infilled frame – X direction).

Demand spectra are provided by INGV [33] in terms of parameters PGA, F_0 and T_C^* (which is multiplied by another coefficient depending on stratigraphic characteristics, C_C , to obtain T_C) for a range of return periods from 30 to 2475 years. For intermediate values of seismic intensity, an interpolation procedure is proposed [32]. Nevertheless, in this study there is the need to extend elastic demand spectra above and below the extreme values, as in [34]. To this aim, the formulations proposed for the interpolation procedure are also used to extrapolate the above mentioned parameters out of the given range of values.

In order to evaluate the influence of material characteristics and element capacity on the seismic response of the case study structure, the following parameters are selected as Random Variables to carry out a sensitivity analysis:

- Concrete compressive strength f_c ;
- Steel yield strength f_y ;
- Infill shear elastic modulus G_w ;
- Chord rotation at yielding in RC members θ_y ;
- Chord rotation at ultimate in RC members θ_u .

A lognormal distribution is assumed for all of the Random Variables. Each distribution is defined through the central (median) value and the Coefficient of Variation (CoV), see Table 1.

For the concrete compressive strength, reference values come from a statistical analysis on the mechanical properties of concrete employed in Italy during 1960s [35]. For the steel yield strength, values are referred to Aq50 steel typology, the most widely spread in Italy during 1960s [36].

The determination of infill material characteristics is affected by high difficulties and uncertainties, and literature does not offer an enough large amount of experimental data. In this study, a median value of 1240 MPa for the shear elastic modulus G_w is adopted, based on wallette tests carried out at the University of Pavia on specimens made up of hollow clay bricks with a void ratio of 42%, selected as representative of typical light non-structural masonry [26]. Nevertheless, there are further infill mechanical characteristics to be determined in order to define, according to the adopted model, the load-displacement relationship of the infill trusses, namely the elastic Young's modulus E_w and the shear cracking stress τ_{cr} . A certain amount of correlation cer-

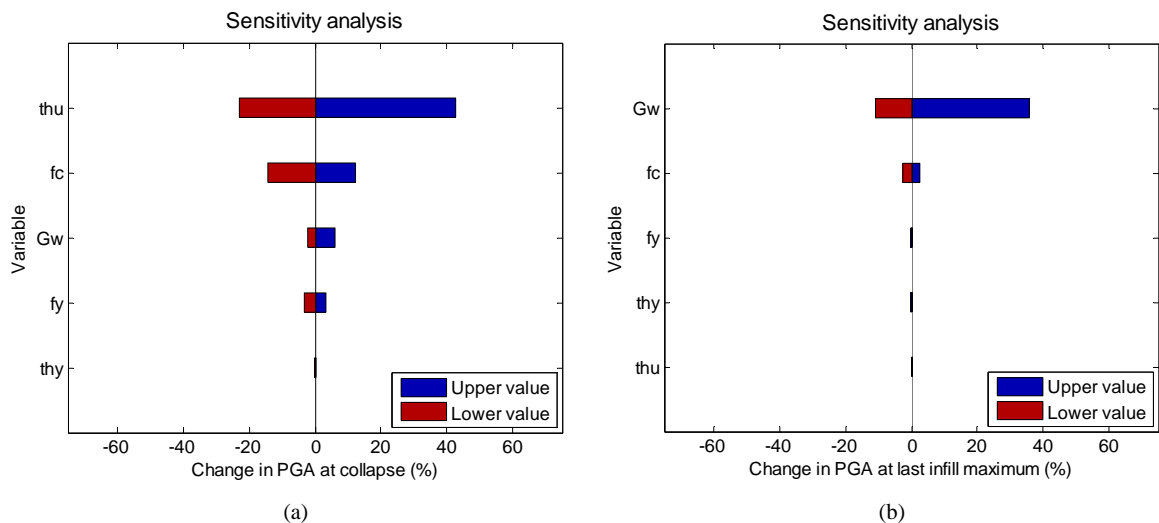


Fig. (5). Results of sensitivity analysis for NC (a) and DL (b) LSs (Uniformly Infilled frame – X direction).

Table 2. Results of pushover and IN2 analyses on the Uniformly Infilled frame in X direction.

Model No.	Modified Variable	Value	T_e [sec]	Δ_{lim} [m]	Δ_{coll} [m]	T_{eff} [sec]	μ_s	$C_{s,max}$ [g]	$C_{s,min}$ [g]	r_u	μ_{lim}	μ_{coll}	R_{lim}	R_{coll}	$S_{ae,lim}$ [g]	$S_{ae,coll}$ [g]	PGA_{lim} [g]	PGA_{coll} [g]	Storey involved in collapse	$\Delta_{PGA,NC}$ [%]	$\Delta_{PGA,lim}$ [%]
1	-	-	0.13	0.01	0.14	0.16	3.04	0.73	0.29	0.39	2.21	23.26	1.33	2.97	0.97	2.17	0.38	0.80	1	-	-
2	f_c	$\mu-1.7\sigma$	0.13	0.01	0.11	0.16	2.89	0.70	0.26	0.37	2.23	18.74	1.34	2.62	0.94	1.84	0.37	0.69	1	-14.4	-2.5
3	"	$\mu+1.7\sigma$	0.13	0.01	0.17	0.16	2.94	0.76	0.30	0.40	2.15	27.21	1.31	3.19	0.99	2.42	0.39	0.90	1	12.3	2.5
4	f_y	$\mu-1.7\sigma$	0.13	0.01	0.14	0.16	3.00	0.73	0.27	0.37	2.21	23.29	1.33	2.88	0.97	2.10	0.38	0.78	1	-3.1	0.2
5	"	$\mu+1.7\sigma$	0.13	0.01	0.14	0.16	3.04	0.73	0.30	0.41	2.20	23.28	1.33	3.08	0.97	2.24	0.38	0.83	1	3.4	-0.2
6	G_w	$\mu-1.7\sigma$	0.16	0.02	0.16	0.20	3.44	0.57	0.23	0.41	2.41	20.95	1.50	3.74	0.85	2.13	0.34	0.79	2	-2.2	-10.8
7	"	$\mu+1.7\sigma$	0.10	0.01	0.14	0.13	2.57	1.00	0.29	0.29	2.21	25.46	1.25	2.01	1.26	2.02	0.52	0.85	1	6.0	35.7
8	θ_y	$\mu-1.7\sigma$	0.13	0.01	0.14	0.16	3.05	0.73	0.29	0.39	2.20	23.19	1.33	2.97	0.97	2.17	0.38	0.80	1	0.0	0.1
9	"	$\mu+1.7\sigma$	0.13	0.01	0.14	0.16	3.02	0.73	0.29	0.39	2.21	23.38	1.33	2.98	0.97	2.17	0.38	0.80	1	0.1	-0.2
10	θ_u	$\mu-1.7\sigma$	0.13	0.01	0.07	0.16	3.04	0.73	0.29	0.39	2.21	12.21	1.33	2.25	0.97	1.64	0.38	0.62	1	-23.0	0.0
11	"	$\mu+1.7\sigma$	0.13	0.01	0.27	0.16	3.04	0.73	0.29	0.39	2.21	44.82	1.33	4.17	0.97	3.04	0.38	1.15	1	42.6	0.0

tainly exists between these parameters, although it is not easy at all to be determined. In this study, a fully correlation is assumed, based on the proposal of the Italian code [39] for the mechanical characteristics of hollow clay brick panels. Hence, the ratio between E_w and G_w is assumed equal to 10/3, whereas τ_{cr} is assumed as linearly dependent on G_w , assuming τ_{cr} equal to 0.3 and 0.4 MPa for G_w equal to 1080 and 1620 MPa, respectively. As far as the modelling of uncertainty in infill mechanical characteristics is concerned, based on some indications from literature [19, 37, 38] a CoV equal to 0.30 is assumed for G_w .

As far as deformations at yielding and ultimate in RC members are concerned, median and CoV values are evaluated starting from the values calculated through the formulations proposed in [23] and using median and CoV values of

the experimental-to-predicted ratio, as illustrated by the author.

SENSITIVITY OF SEISMIC CAPACITY TO MATERIAL AND MODEL VARIABLES

Based on the assumed Random Variables, a sensitivity analysis is carried out to investigate the influence of each variable on the seismic capacity of the case study structure. To this aim, two models are generated for each random variable assuming median-minus-1.7-standard-deviation and median-plus-1.7-standard-deviation values for the considered variable, and median values for the remaining variables. In addition to these analyses, another one is carried out assuming median values for all of the variables (Model #1).

In the following, obtained results are presented and discussed for Uniformly infilled, Pilotis and Bare frames, in

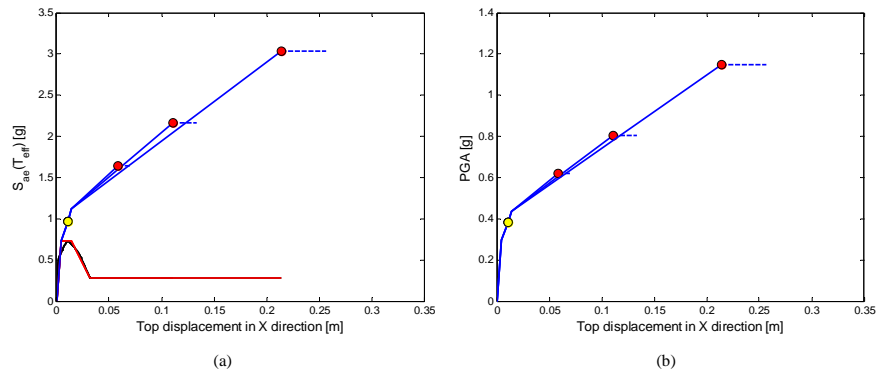


Fig. (6). Obtained (black) and idealized (red) pushover curves and IN2 (blue) curves in terms of $S_{ae}(T_{eff})$ (a) and PGA (b) for Models #10, #1 and #11: Lower, Median and Upper values for θ_u (Uniformly Infilled frame – X direction).

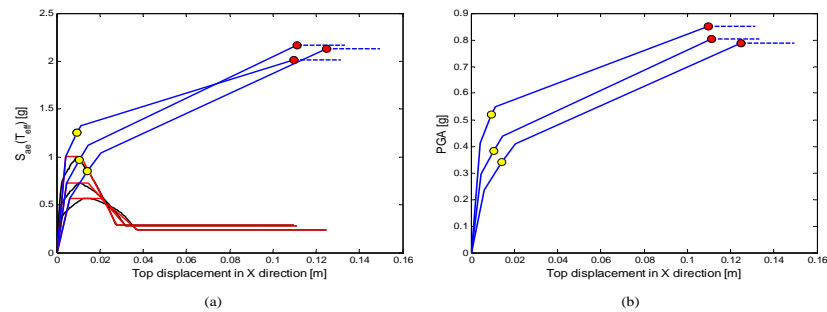


Fig. (7). Obtained (black) and idealized (red) pushover curves and IN2 (blue) curves in terms of $S_{ae}(T_{eff})$ (a) and PGA (b) for Models #6, #1 and #7: Lower, Median and Upper values for infill mechanical characteristics (Uniformly Infilled frame – X direction).

both longitudinal (X) and transverse (Y) directions and at Damage Limitation and Near Collapse Limit States. Obtained and idealized pushover curves and corresponding IN2 curves – both in terms of $S_{ae}(T_{eff})$ and PGA – are reported for different models, in black, red and blues colors, respectively. The $[S_d, S_{ae}(T_{eff})]$ and $[S_d, PGA]$ points on IN2 curves corresponding to DL and NC Limit State are reported as yellow and red circles, respectively. The dashed horizontal branch in IN2 curves after the NC Limit State conventionally represents the structural failure [18]. The schematic construction of an IN2 curve, both in terms of $S_{ae}(T_{eff})$ and PGA, is illustrated in Fig. (3), referring to Model #1 of Uniformly infilled frame in X direction.

It is to be noted that the influence of each single variable, which will be illustrated through the sensitivity analysis, not only depends on the influence of the variable on the seismic response, but also depends on the dispersion assumed for that variable through the assigned CoV, which leads to consider – as Lower and Upper limits – values more or less distant from the central (median) value.

Pushover and IN2 curves for Model #1 of Uniformly infilled frame in X direction are reported in Fig. (4a), deformed shape at NC is reported in Fig. (4b). Results of sensitivity analysis for NC and DL Limit States are reported in Figs. (5a and 5b), respectively. Numerical results are also reported in Table 2.

The building collapses under a soft-storey mechanism at the 1st storey in all cases except when a lower value is assumed for infill mechanical characteristics: in this case there is a soft-storey mechanism at the 2nd storey.

The sensitivity analysis shows that θ_u has the highest influence on the PGA at collapse. This is clearly due to the fact the displacement capacity at collapse is directly given by the rotational capacity of columns, given the soft-storey collapse mechanism. Thus, an increase in θ_u results in an increase in the MDOF displacement capacity at first RC element collapse ($D_{collapse}$), that is, an increase in the ductility at first RC element collapse ($\mu_{collapse}$), leading to higher values of $S_{ae}(T_{eff})$ leading to collapse ($S_{ae,collapse}$) (see Fig. (6a)) and, hence, of the corresponding PGA ($PGA_{collapse}$) (see Fig. (6b)). Vice versa if θ_u decreases, f_c influences the collapse capacity through the value of θ_u : given equal the axial load, as f_c increases the axial load ratio decreases and the rotational capacity of the columns increases, thus leading to a higher global ductility. Vice versa if f_c decreases. When E_w , G_w and τ_{cr} increase, several effects can be observed: the increase in stiffness and strength leads to a lower T_{eff} and a higher maximum inelastic acceleration capacity ($C_{s,max}$); the minimum inelastic acceleration capacity ($C_{s,min}$) does not change significantly, hence a detrimental decrease in r_u is observed. $D_{collapse}$ does not change significantly, but the decrease in the displacement at yielding of the equivalent SDOF system (S_{dy}) leads to an increase in $\mu_{collapse}$. These effects globally lead to a much lower value of the strength reduction factor at collapse ($R_{collapse}$); however, the higher base shear capacity $C_{s,max}$ leads to a value of $S_{ae,collapse}$ only slightly lower, compared with Model #1 (see Fig. (7a)). Nevertheless, due to the decrease in T_{eff} , this lower value of $S_{ae,collapse}$ corresponds to a higher value of $PGA_{collapse}$ (see Fig. (7b)). Opposite observations, compared with the previous case, can be made when E_w , G_w and τ_{cr} increase, thus resulting in a lower $PGA_{collapse}$ (see Fig. (7b)). When E_w ,

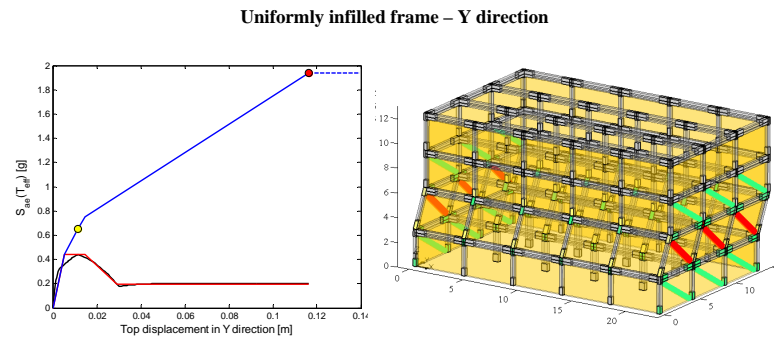


Fig. (8). Obtained (black) and idealized (red) pushover curves and IN2 (blue) curve (a); deformed shape and element damage at NC (b) (Model #1 – Uniformly Infilled frame – Y direction).

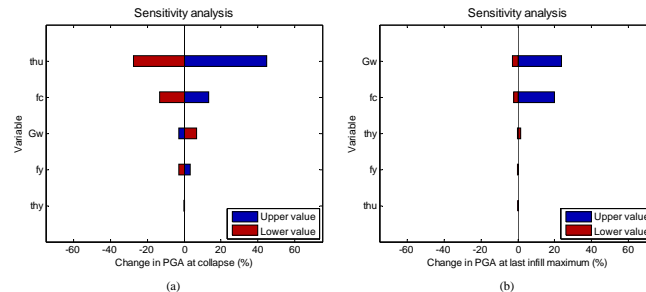


Fig. (9). Results of sensitivity analysis for NC (a) and DL (b) LSs (Uniformly Infilled frame – Y direction).

G_w and τ_{cr} decrease, the building collapses under a soft-storey mechanism at the 2nd storey instead of the 1st one. From a qualitative standpoint, a higher infill strength leads to a *more uniform distribution* of maximum interstorey shear strength along the height of the building: more uniform this distribution, higher the probability to have a soft storey mechanism at the 1st storey, where the shear demand is higher. For the same reason, a lower infill strength may lead to the formation of a soft-storey mechanism at the 2nd storey instead of the 1st one. At the 2nd storey the strength of RC columns is lower, thus leading to a lower value of the residual strength $C_{s,min}$, which is essentially due to the contribution of RC frame due to the failure of infills. The only parameter significantly influenced by f_y is the residual base shear $C_{s,min}$. An increase in f_y results in a higher $C_{s,min}$, thus leading to a higher r_u . This beneficial effect results in a higher $R_{collapse}$, leading to higher values of $S_{ae,collapse}$ and $PGA_{collapse}$. Vice versa if f_y decreases. The rotation at yielding θ_y has no significant influence on PGA capacity at collapse.

As far as PGA capacity at last infill maximum is concerned, from a qualitative standpoint, the same trend observed for PGA capacity at collapse with the infill mechanical characteristics are observed. Hence, a beneficial effect of an increase in stiffness and strength is observed. When f_c increases, $C_{s,max}$ increases too, mainly due to the higher contribution of RC columns – due to their higher stiffness and strength – to the maximum base shear, which corresponds to the attainment of the maximum strength in the infills in the storey involved in collapse and is attained for the same displacement. On the whole, the change in f_c does not influence significantly the strength reduction factor at last infill maximum (R_{lim}), but the increase in $C_{s,max}$ leads to higher values of $S_{ae}(T_{eff})$ leading to last infill maximum ($S_{ae,lim}$) and of the corresponding PGA (PGA_{lim}). Vice versa if f_c decreases.

Remaining parameters do not have a significant influence on PGA_{lim} .

Pushover and IN2 curves for Model #1 of Uniformly infilled frame in Y direction are reported in Fig. (8a), deformed shape at NC is reported in Fig. (8b). Results of sensitivity analysis for NC and DL Limit States are reported in Figs. (9a and 9b), respectively. Numerical results are also reported in Table 3.

The building collapses under a soft-storey mechanism at the 2nd storey in all cases except when a lower value is assumed for f_c and when an upper value is assumed for infill mechanical characteristics. In these cases, the storey involved by the collapse mechanism is the 1st one. As already highlighted, a lower stiffness and/or strength of the RC structure and a higher strength of infills lead to a *more uniform distribution* of maximum interstorey shear strength along the height of the building: more uniform is this distribution, higher is the probability to have a soft storey mechanism at the 1st storey, where the shear demand is higher. The attention has to be focused not only on the *strength* of the RC structure, but also on its *stiffness*. As a matter of fact, in infilled RC frames the soft-storey collapse mechanism generally takes place at the storey where – under the given distribution of lateral forces – the maximum ratio between the interstorey shear demand and the interstorey shear strength takes place. From a qualitative standpoint, the latter value is provided by the strength contribution of infills, corresponding to their maximum resistance and attained for rather low displacement values, and by a contribution of RC columns, which at this stage of behavior have not developed their entire strength yet. Hence, the latter contribution is significantly influenced by *stiffness* of RC elements, and not only by their *strength*. As already illustrated for the X direction, when E_w , G_w and τ_{cr} increase, several effects are observed,

Table 3. Results of pushover and IN2 analyses on the Uniformly Infilled frame in Y direction.

Model No.	Modified Variable	Value	T_{el} [sec]	Δ_{lim} [m]	Δ_{coll} [m]	T_{eff} [sec]	μ_s	$C_{s,max}$ [g]	$C_{s,min}$ [g]	r_u	μ_{lim}	μ_{coll}	R_{lim}	R_{coll}	$S_{ae,lim}$ [g]	$S_{ae,coll}$ [g]	PGA _{lim} [g]	PGA _{coll} [g]	Storey involved in collapse	$\Delta_{PGA,NC}$ [%]	$\Delta_{PGA,lim}$ [%]
1	-	-	0.17	0.02	0.15	0.22	2.81	0.44	0.20	0.45	2.20	22.29	1.48	4.39	0.65	1.94	0.27	0.72	2	-	-
2	f_c	$\mu-1.7\sigma$	0.18	0.01	0.11	0.22	2.94	0.43	0.21	0.48	2.23	16.81	1.49	3.87	0.64	1.65	0.26	0.62	1	-13.4	-2.3
3	"	$\mu+1.7\sigma$	0.17	0.02	0.18	0.22	3.04	0.45	0.21	0.46	3.04	26.71	1.77	4.95	0.79	2.22	0.32	0.82	2	13.2	19.8
4	f_y	$\mu-1.7\sigma$	0.17	0.02	0.15	0.22	2.86	0.44	0.19	0.42	2.19	22.26	1.48	4.25	0.65	1.88	0.27	0.70	2	-2.8	0.0
5	"	$\mu+1.7\sigma$	0.17	0.02	0.15	0.22	2.80	0.44	0.21	0.48	2.21	22.44	1.48	4.57	0.65	2.01	0.27	0.75	2	3.4	0.0
6	G_w	$\mu-1.7\sigma$	0.22	0.02	0.16	0.27	3.63	0.33	0.20	0.59	2.86	21.08	1.91	6.31	0.63	2.08	0.26	0.77	2	6.8	-3.1
7	"	$\mu+1.7\sigma$	0.14	0.01	0.14	0.18	2.56	0.61	0.22	0.37	2.15	22.93	1.35	3.09	0.82	1.88	0.33	0.70	1	-2.8	23.6
8	θ_y	$\mu-1.7\sigma$	0.17	0.02	0.15	0.22	2.83	0.44	0.20	0.45	2.26	22.33	1.50	4.40	0.66	1.94	0.27	0.72	2	0.2	1.4
9	"	$\mu+1.7\sigma$	0.17	0.02	0.15	0.22	2.80	0.44	0.20	0.45	2.20	22.30	1.48	4.40	0.65	1.94	0.27	0.72	2	-0.1	0.0
10	θ_u	$\mu-1.7\sigma$	0.17	0.02	0.08	0.22	2.81	0.44	0.20	0.44	2.20	12.05	1.48	3.09	0.65	1.36	0.27	0.52	2	-27.5	0.0
11	"	$\mu+1.7\sigma$	0.17	0.02	0.29	0.22	2.81	0.44	0.20	0.45	2.20	42.27	1.48	6.59	0.65	2.90	0.27	1.04	2	44.9	0.0

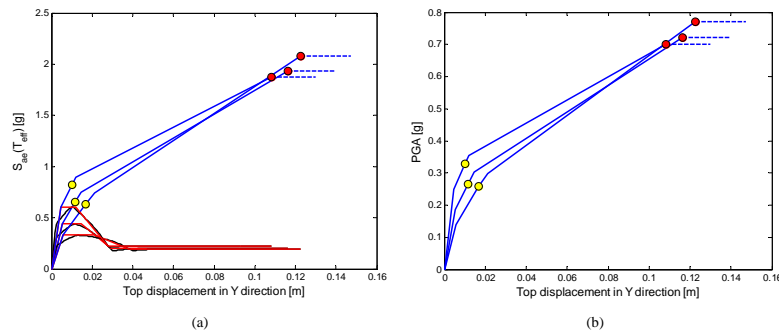


Fig. (10). Obtained (black) and idealized (red) pushover curves and IN2 (blue) curves in terms of $S_{ae}(T_{eff})$ (a) and PGA (b) for Models #6, #1 and #7: Lower, Median and Upper values for infill mechanical characteristics (Uniformly Infilled frame – Y direction).

including the decrease in r_u , globally leading to a lower value of $R_{collapse}$; this effect may or may not be counterbalanced by the higher $C_{s,max}$. Opposite to X direction, in the case of Y direction the effect of decrease in $R_{collapse}$ prevails over the increase in $C_{s,max}$, leading to a lower $S_{ae,collapse}$ see Fig. (10a) and also to a lower $PGA_{collapse}$ see Fig. (10b). Vice versa when lower values of infill mechanical characteristics are considered. The influence of remaining parameter is quite similar to X direction.

As far as PGA capacity at last infill maximum is concerned, opposite to PGA capacity at collapse, a beneficial effect of the increase in E_w , G_w and τ_{cr} is observed; as a matter of fact, see Fig. (10a), the beneficial effect of a higher strength (higher value of $C_{s,max}$) on the capacity – in terms of $S_{ae}(T_{eff})$ – is more important when the ductility capacity is in a lower range of values (e.g., DL limit state). If the ductility capacity is, on average, higher (e.g., NC limit state), the detrimental effect of a more brittle behavior (lower value of r_u) on the corresponding capacity – expressed as $S_{ae}(T_{eff})$ – tends

to prevail. This trend is reflected by the decrease in the slope of IN2 curves when the infill mechanical characteristics assume upper values. Based on these observations, when E_w , G_w and τ_{cr} decrease a lower PGA capacity at last infill maximum is expected. Nevertheless, due to the full correlation assumed between the infill mechanical characteristics, in the adopted model a lower strength and a lower secant-to-maximum stiffness are observed at the same time, resulting in a higher displacement capacity at this Limit State. This effect partially counterbalances the detrimental effect of lower initial stiffness and strength, leading to a only slightly lower PGA capacity at last infill maximum. Remaining parameters do not have a significant influence on the PGA capacity at last infill maximum.

Pushover and IN2 curves for Model #1 of Pilotis frame in X direction are reported in Fig. (11a), deformed shape at NC is reported in Fig. (11b). Results of sensitivity analysis for NC and DL Limit States are reported in Figs. (12a and

Pilotis frame – X direction

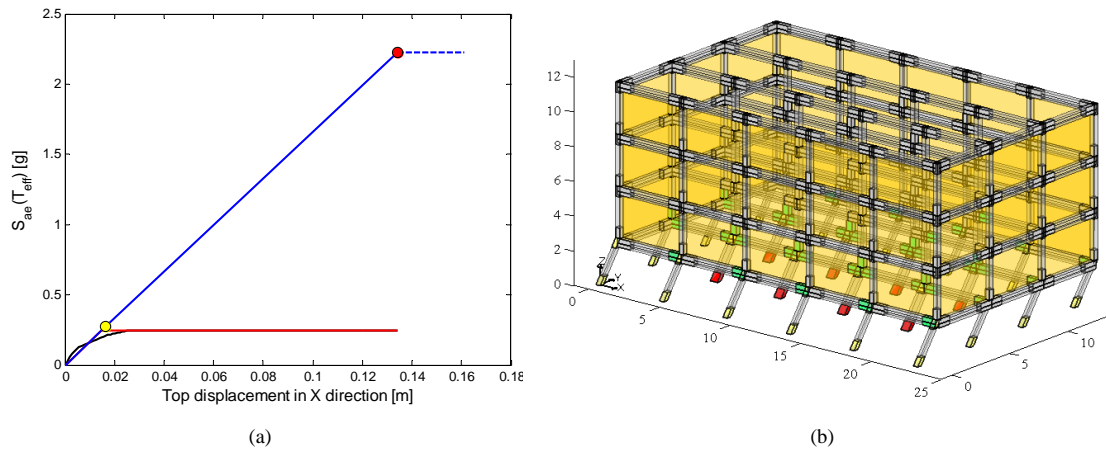


Fig. (11). Obtained (black) and idealized (red) pushover curves and IN2 (blue) curve (a); deformed shape and element damage at NC (b) (Model #1 – Pilotis frame – X direction).

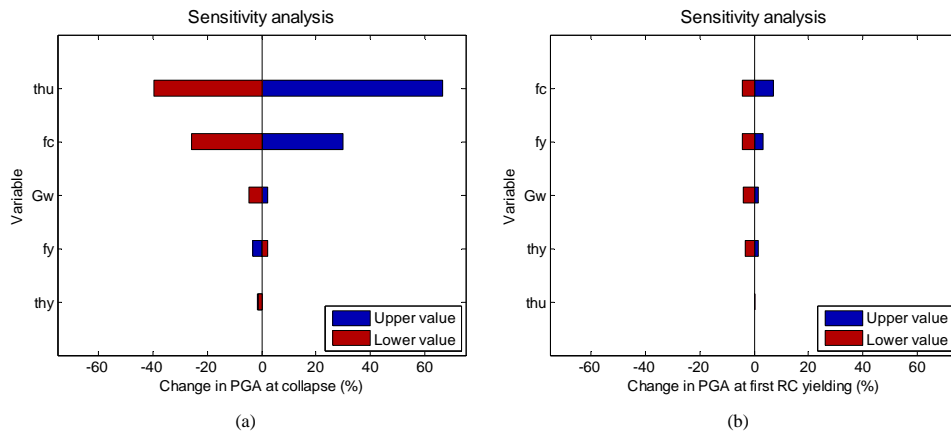


Fig. (12). Results of sensitivity analysis for NC (a) and DL (b) LSs (Pilotis frame – X direction).

12b), respectively. Numerical results are also reported in Table 4.

The building collapses under a soft-storey mechanism at the 1st storey, where infills are not present, in all cases. Similar to previous cases, the sensitivity analysis shows that θ_u has the highest influence on the PGA at collapse through $\Delta_{collapse}$, that is, through $\mu_{collapse}$, directly influencing $S_{ae,collapse}$ and, hence, $PGA_{collapse}$. f_c also influences the collapse capacity through the value of θ_u , as already illustrated. The beneficial effect of the increase in E_w , G_w and τ_{cr} is due to fact that in this case the distribution of lateral forces is closer to a *uniform* distribution (this is reflected by the decrease in the modal participation factor Γ), leading to a higher initial stiffness of the pushover curve, resulting in lower values of T_{eff} and S_{dy} , whereas the displacement capacity $\Delta_{collapse}$ does not change significantly and, hence, $\mu_{collapse}$ increases, leading to higher values of $S_{ae,collapse}$ and $PGA_{collapse}$. An increase in f_y leads to a higher $C_{s,max}$, but also to higher values of S_{dy} and T_{eff} . Given practically equal the displacement capacity $\Delta_{collapse}$, $\mu_{collapse}$ decreases and this effect prevails over the beneficial increase in $C_{s,max}$, leading to lower values of $S_{ae,collapse}$ and $PGA_{collapse}$. Vice versa if f_y decreases. The rotation at

yielding θ_y has no significant influence on PGA capacity at collapse.

At DL Limit State, when f_c decreases, both S_{dy} and the MDOF displacement capacity at first yielding in RC members (Δ_{fRCy}) increase, but the former effect prevails on the latter, leading to a lower value of the ductility at first yielding in RC members (μ_{fRCy}) and, hence, of the corresponding strength reduction factor (R_{fRCy}). Moreover, the detrimental effect of a decrease in f_c on $S_{ae,fRCy}$ is also given by the decrease in $C_{s,max}$. Vice versa when f_c increases. Opposite to the case of f_c , when f_y decreases both S_{dy} and Δ_{fRCy} decrease, but the former effect prevails on the latter, leading to a higher value of μ_{fRCy} and, hence, of R_{fRCy} . Nevertheless, the detrimental effect of the decrease in $C_{s,max}$ prevails. Vice versa when f_y increases. Contrary to expectations, an increase in θ_y does not have a great beneficial influence on $S_{ae,fRCy}$ and, hence, on the PGA leading to first yielding in RC members (PGA_{fRCy}). This is due to the fact that when θ_y increases the displacement capacity Δ_{fRCy} increases, but S_{dy} increases too, leading to a not much different value of μ_{fRCy} and, hence, of R_{fRCy} . An increase in E_w , G_w and τ_{cr} , as already illustrated,

Table 4. Results of pushover and IN2 analyses on the Pilotis frame in X direction.

Model No.	Modified Variable	Value	T_d [sec]	Δ_{RCy} [m]	Δ_{coll} [m]	T_{eff} [sec]	C_{amax} [g]	μ_{RCy}	μ_{coll}	R_{RCy}	R_{coll}	$S_{ac,RCy}$ [g]	$S_{ac,coll}$ [g]	PGA_{RCy} [g]	PGA_{coll} [g]	Storey involved in collapse	Δ_{PGANC} [%]	$\Delta_{PGA_{RCy}}$ [%]
1	-	-	0.39	0.02	0.14	0.49	0.24	1.12	9.15	1.12	9.11	0.27	2.22	0.16	0.82	1	-	-
2	f_c	$\mu-1.7\sigma$	0.46	0.02	0.11	0.57	0.22	1.01	5.83	1.01	5.83	0.22	1.30	0.15	0.61	1	-25.7	-4.3
3	"	$\mu+1.7\sigma$	0.35	0.02	0.17	0.43	0.26	1.33	13.92	1.33	11.52	0.34	2.96	0.17	1.06	1	30.0	7.0
4	f_y	$\mu-1.7\sigma$	0.38	0.02	0.14	0.48	0.23	1.15	10.14	1.15	9.83	0.27	2.28	0.15	0.84	1	2.1	-4.5
5	"	$\mu+1.7\sigma$	0.40	0.02	0.14	0.52	0.26	1.05	7.89	1.05	7.89	0.27	2.04	0.16	0.79	1	-3.4	3.2
6	G_w	$\mu-1.7\sigma$	0.39	0.02	0.14	0.52	0.24	1.02	8.18	1.02	8.18	0.25	2.00	0.15	0.78	1	-4.6	-3.9
7	"	$\mu+1.7\sigma$	0.38	0.02	0.14	0.48	0.24	1.16	9.58	1.16	9.35	0.28	2.28	0.16	0.84	1	2.3	1.4
8	θ_y	$\mu-1.7\sigma$	0.38	0.02	0.14	0.50	0.24	1.06	8.86	1.06	8.86	0.26	2.16	0.15	0.81	1	-1.3	-3.4
9	"	$\mu+1.7\sigma$	0.39	0.02	0.14	0.50	0.24	1.12	8.79	1.12	8.79	0.27	2.15	0.16	0.81	1	-1.5	1.4
10	θ_u	$\mu-1.7\sigma$	0.39	0.02	0.07	0.49	0.24	1.12	4.71	1.12	4.71	0.27	1.15	0.16	0.49	1	-39.6	0.0
11	"	$\mu+1.7\sigma$	0.39	0.02	0.27	0.49	0.24	1.12	17.80	1.12	15.92	0.27	3.88	0.16	1.36	1	66.6	0.0

Pilotis frame – Y direction

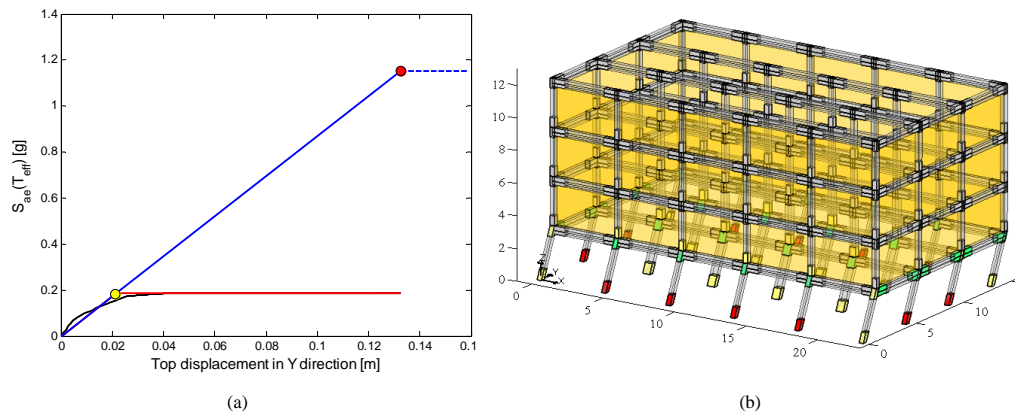


Fig. (13). Obtained (black) and idealized (red) pushover curves and IN2 (blue) curve (a); deformed shape and element damage at NC (b) (Model #1 – Pilotis frame – Y direction).

leads to a higher initial stiffness of the pushover curve, resulting in lower values of S_{dy} but also of Δ_{RCy} . These two effects counterbalance each other, globally leading to no significant change in μ_{RCy} and, hence, in R_{RCy} . When E_w , G_w and τ_{cr} decrease, both S_{dy} and Δ_{RCy} increase, but the former effect prevails on the latter, leading to a lower value of μ_{RCy} and, hence, of R_{RCy} .

Pushover and IN2 curves for Model #1 of Pilotis frame in Y direction are reported in Fig. (13a), deformed shape at NC is reported in Fig. (13b). Results of sensitivity analysis for NC and DL Limit States are reported in Figs. (14a and 14b), respectively. Numerical results are also reported in Table 5.

Also in this direction, the building always collapses under a soft-storey mechanism at the 1st storey, where infills

are not present. The same considerations made for X direction can be reported for all parameters, except infill mechanical characteristics since in this direction their influence – which was already not particularly significant in X direction – is absolutely negligible, due to the lower number of infill panels in Y direction.

At first RC yielding, from a qualitative standpoint, analyzed parameters influence the PGA capacity by the same way in Y direction, compared with X direction. Again, the only exception is for infill mechanical characteristics, whose influence on the seismic behavior in Y direction is absolutely negligible.

Pushover and IN2 curves for Model #1 of Bare frame in X direction are reported in Fig. (15a), deformed shape at NC is reported in Fig. (15b). Results of sensitivity analysis for

Table 5. Results of pushover and IN2 analyses on the Pilotis frame in Y direction.

Model No.	Modified Variable	Value	T_e [sec]	Δ_{RCY} [m]	Δ_{coll} [m]	T_{eff} [sec]	$C_{s,max}$ [g]	μ_{RCY}	μ_{coll}	R_{RCY}	R_{coll}	$S_{ae,RCY}$ [g]	$S_{ae,coll}$ [g]	PGA_{RCY} [g]	PGA_{coll} [g]	Storey involved in collapse	ΔPGA_{NC} [%]	ΔPGA_{RCY} [%]
1	-	-	0.54	0.02	0.14	0.68	0.19	0.98	6.18	0.99	6.18	0.18	1.15	0.15	0.63	1	-	-
2	f_c	$\mu-1.7\sigma$	0.61	0.02	0.11	0.77	0.17	0.89	3.94	0.91	3.94	0.16	0.68	0.15	0.47	1	-25.8	-2.3
3	"	$\mu+1.7\sigma$	0.49	0.02	0.17	0.61	0.20	1.10	8.90	1.10	8.90	0.21	1.74	0.16	0.80	1	26.1	4.4
4	f_y	$\mu-1.7\sigma$	0.53	0.02	0.14	0.65	0.18	1.04	7.17	1.04	7.17	0.18	1.26	0.14	0.66	1	3.5	-4.3
5	"	$\mu+1.7\sigma$	0.55	0.02	0.14	0.71	0.20	0.94	5.38	0.95	5.38	0.19	1.07	0.16	0.62	1	-2.8	5.9
6	G_w	$\mu-1.7\sigma$	0.55	0.02	0.14	0.69	0.19	0.96	6.03	0.97	6.03	0.18	1.13	0.15	0.63	1	-0.6	-0.3
7	"	$\mu+1.7\sigma$	0.54	0.02	0.14	0.69	0.19	0.95	6.05	0.96	6.05	0.18	1.13	0.15	0.63	1	-0.4	-1.1
8	θ_y	$\mu-1.7\sigma$	0.54	0.02	0.14	0.68	0.19	0.97	6.24	0.98	6.24	0.18	1.16	0.15	0.64	1	0.4	-1.3
9	"	$\mu+1.7\sigma$	0.55	0.02	0.14	0.68	0.19	1.01	6.14	1.01	6.14	0.19	1.15	0.15	0.63	1	-0.2	2.4
10	θ_u	$\mu-1.7\sigma$	0.54	0.02	0.07	0.68	0.19	0.98	3.19	0.99	3.19	0.18	0.60	0.15	0.38	1	-39.4	0.0
11	"	$\mu+1.7\sigma$	0.54	0.02	0.27	0.68	0.19	0.98	11.99	0.99	11.99	0.18	2.24	0.15	1.05	1	65.5	0.0

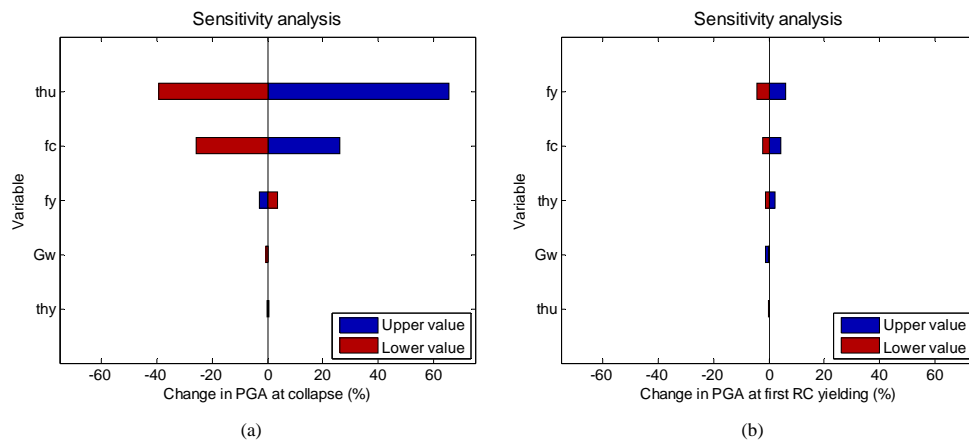


Fig. (14). Results of sensitivity analysis for NC (a) and DL (b) LS (Pilotis frame – Y direction).

NC and DL Limit States are reported in Figs. (16a and 16b), respectively. Numerical results are also reported in Table 6.

The building collapses under a soft-storey mechanism at the 3rd storey in all cases. The formation of a soft-storey mechanism also without the presence of infill panels is likely to occur in such a building, which has not been designed for seismic loads and, obviously, does not comply with Capacity Design principles such as weak beam/strong column condition. In the Uniformly infilled frame the soft-storey mechanism occurs at one storey at the bottom (1st or 2nd storey), thus confirming the well-known phenomenon of concentration of displacement demand in bottom storeys in this kind of structures, due to the more uniform distribution of strength along the height, whereas in the Bare frame this mechanism occurs at the 3rd storey, where a decrease in column dimension (and strength) is observed, due to the Gravity Load Design procedure. Again, the sensitivity analysis

shows that θ_u has the highest influence on the PGA at collapse through $\Delta_{collapse}$, that is, through $\mu_{collapse}$, directly influencing $S_{ae,collapse}$ and, hence, $PGA_{collapse}$. Similarly, f_c also influences the collapse capacity through the value of θ_u , as already illustrated. When f_y increases, the displacement capacity $\Delta_{collapse}$ does not change significantly, but S_{dy} increases, thus leading to a reduction in $\mu_{collapse}$. Nevertheless, this effect is counterbalanced by the increase in $C_{s,max}$, resulting in no significant change in $S_{ae,collapse}$ and $PGA_{collapse}$. Vice versa if f_y decreases. The rotation at yielding θ_y has no significant influence on PGA capacity at collapse.

At DL Limit State, when f_y decreases both S_{dy} and Δ_{RCY} decrease, but (contrary to the Pilotis frame) the latter effect prevails on the former, leading to a lower value of μ_{RCY} and, hence, of R_{RCY} . Moreover, the decrease in $C_{s,max}$ leads to a further decrease in $S_{ae,RCY}$ and PGA_{RCY} . Vice versa when f_y increases. For the same reasons illustrated for the Pilotis

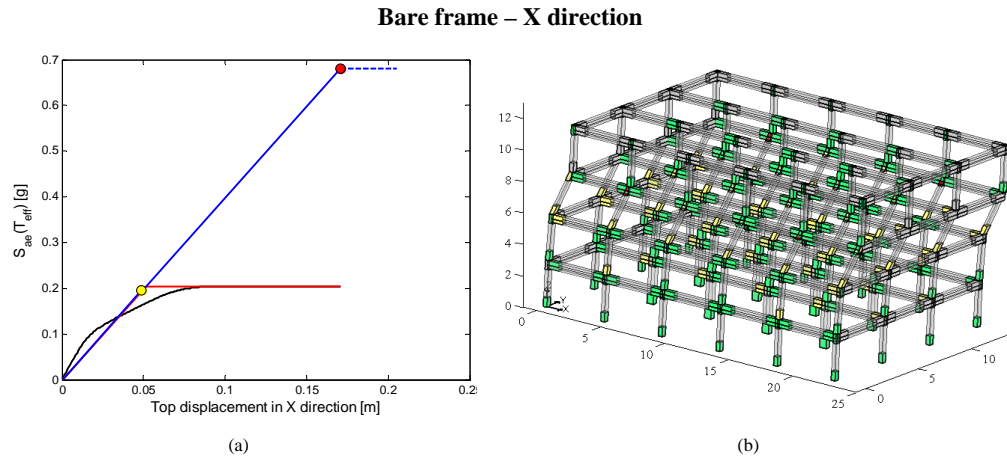


Fig. (15). Obtained (black) and idealized (red) pushover curves and IN2 (blue) curve (a); deformed shape and element damage at NC (b) (Model #1 – Bare frame – X direction).

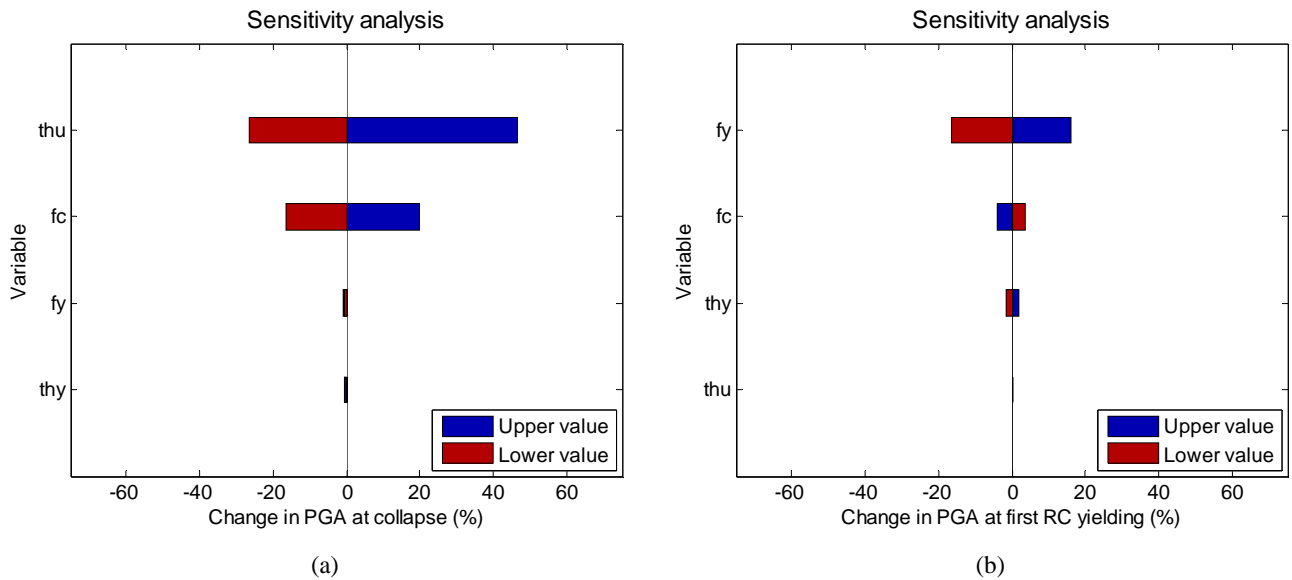


Fig. (16). Results of sensitivity analysis for NC (a) and DL (b) LSs (Bare frame – X direction).

frame, when f_c decreases a lower value of $S_{ae,RCy}$ is observed. Nevertheless, the increase in T_{eff} counterbalances this effect, leading to a slightly higher capacity in terms of PGA. Vice versa when f_c increases. Again, an increase in θ_y does not have a great beneficial influence on $S_{ae,RCy}$ and, hence, on PGA_{RCy} , for the same reasons above illustrated for the Pilotis frame: when θ_y increases, the displacement capacity Δ_{RCy} increases, but S_{dy} increases too, leading to a not much different value of μ_{RCy} and, hence, of R_{RCy} . As expected, θ_u has no significant influence on PGA capacity at this Limit State.

Pushover and IN2 curves for Model #1 of Bare frame in Y direction are reported in Fig. (17a), deformed shape at NC is reported in Fig. (17b). Results of sensitivity analysis for NC and DL Limit States are reported in Fig. (18a and 18b), respectively. Numerical results are also reported in Table 7.

In this direction, the building always collapses under a global mechanism involving all of the four storeys. This col-

lapse mechanism is certainly strongly influenced by the structural configuration, where beams in transverse direction are present only in external frames. A strong difference with the Uniformly infilled and the Pilotis frame, where the presence of infill panels forces the collapse mechanism to develop only in one storey (at the bottom), is noted. As far as the influence of single variables is concerned, the main influence of displacement capacity through θ_u and, indirectly, through f_c is observed again. The decrease in f_y leads to a lower value of S_{dy} , whereas the displacement capacity $\Delta_{collapse}$ does not change significantly, thus leading to a higher $\mu_{collapse}$ and, hence, to a higher $R_{collapse}$. However, this effect is counterbalanced by a lower $C_{s,max}$, thus leading to a only slightly higher $PGA_{collapse}$.

At DL Limit State, similar to PGA capacity at collapse, it is observed that the decrease in f_y leads to a lower value of S_{dy} , whereas the displacement capacity Δ_{RCy} also decreases but in a lower measure, thus leading to a higher μ_{RCy} and, hence, to a higher R_{RCy} . However, this effect is counterbal-

Table 6. Results of pushover and IN2 analyses on the Bare frame in X direction.

Model No.	Modified Variable	Value	T_d [sec]	Δ_{RCy} [m]	Δ_{coll} [m]	T_{eff} [sec]	$C_{s,max}$ [g]	μ_{RCy}	μ_{coll}	R_{RCy}	R_{coll}	S_{sefRCy} [g]	$S_{sefcoll}$ [g]	PGA_{RCy} [g]	PGA_{coll} [g]	Storey involved in collapse	$\Delta_{PGA,NC}$ [%]	$\Delta_{PGA,RCy}$ [%]
1	-	-	0.76	0.06	0.22	1.01	0.20	0.95	3.33	0.95	3.33	0.19	0.68	0.22	0.57	3	-	-
2	f_c	$\mu-1.7\sigma$	0.99	0.08	0.20	1.18	0.19	0.89	2.38	0.90	2.38	0.17	0.46	0.23	0.48	3	-16.3	3.6
3	"	$\mu+1.7\sigma$	0.67	0.05	0.24	0.85	0.21	1.03	4.80	1.03	4.80	0.22	1.02	0.21	0.68	3	19.8	-4.1
4	f_y	$\mu-1.7\sigma$	0.74	0.05	0.22	1.00	0.19	0.81	3.54	0.82	3.54	0.16	0.68	0.18	0.57	3	-0.4	-16.5
5	"	$\mu+1.7\sigma$	0.78	0.08	0.22	1.04	0.22	1.04	3.00	1.04	3.00	0.23	0.65	0.25	0.57	3	-0.8	16.1
6	θ_y	$\mu-1.7\sigma$	0.75	0.06	0.22	1.00	0.20	0.94	3.37	0.95	3.37	0.19	0.69	0.22	0.57	3	0.2	-1.5
7	"	$\mu+1.7\sigma$	0.77	0.07	0.22	1.02	0.20	0.95	3.24	0.96	3.24	0.20	0.66	0.22	0.57	3	-0.6	1.8
8	θ_u	$\mu-1.7\sigma$	0.76	0.06	0.15	1.01	0.20	0.95	2.22	0.95	2.22	0.19	0.45	0.22	0.42	3	-26.5	0.0
9	"	$\mu+1.7\sigma$	0.76	0.06	0.36	1.01	0.20	0.95	5.50	0.95	5.50	0.19	1.12	0.22	0.84	3	46.3	0.0

Bare frame – Y direction

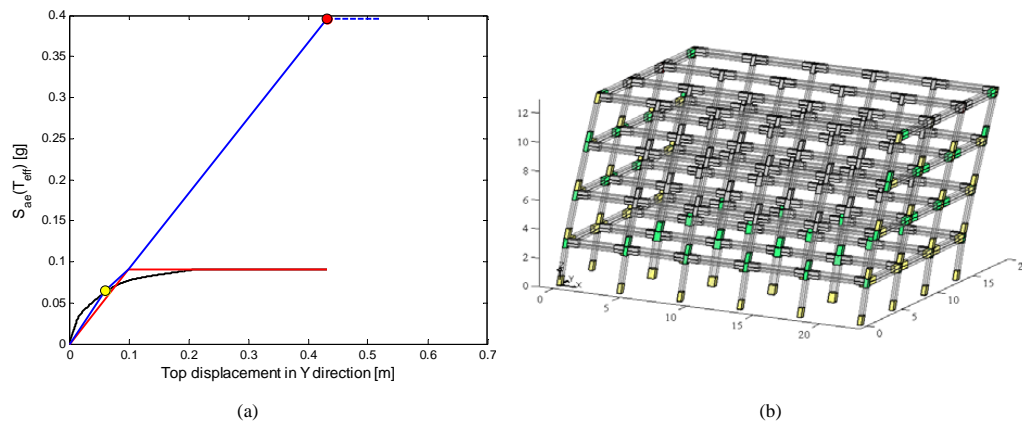


Fig. (17). Obtained (black) and idealized (red) pushover curves and IN2 (blue) curve (a); deformed shape and element damage at NC (b) (Model #1 – Bare frame – Y direction).

anced by a lower $C_{s,max}$, thus leading to a only slightly higher PGA_{RCy} . As far as θ_y is concerned, it is observed again that when θ_y increases, the displacement capacity Δ_{RCy} increases, but S_{dy} increases too, thus counterbalancing the former effect. Vice versa when θ_y decreases. For the same reasons, also f_c has a negligible influence on PGA capacity at first RC yielding.

COMPARISON BETWEEN DIFFERENT INFILL CONFIGURATIONS AND LOADING DIRECTIONS

In this Section, the influence of different infill configurations on the seismic capacity of the case study building is evaluated. To this aim, IN2 curves are compared, always referring to the models where a median values are assumed for all of the variables. The comparison is carried out in both directions.

Uniformly Infilled Frame – Comparison Between X and Y Directions

If a comparison is carried out between the seismic capacity in X and Y directions for Model #1 see Figs. (19a and 19b) it is observed how the displacement capacity does not change significantly and the beneficial effect of a higher strength, both maximum ($C_{s,max}$) and residual ($C_{s,min}$), leads to higher PGA capacities in X direction, both at collapse and last infill maximum. The higher strength in X direction compared with Y direction is due (i) to the larger amount of infill panels in X direction and (ii) to the orientation of column elements, which, following the parallel plane frame configuration, provide a higher strength in X direction.

Pilotis Frame – Comparison Between X and Y Directions

If a comparison is carried out between the seismic capacity in X and Y directions for Model #1 see Fig. (20a and 20b) it is observed how the displacement capacity at collapse

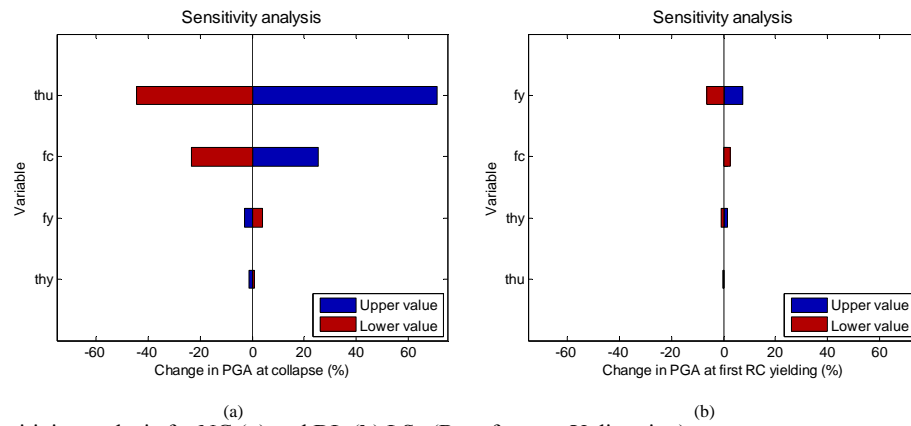


Fig. (18). Results of sensitivity analysis for NC (a) and DL (b) LSs (Bare frame – Y direction).

Table 7. Results of pushover and IN2 analyses on the Bare frame in Y direction.

Model No.	Modified Variable	Value	T_{el} [sec]	Δ_{RCy} [m]	Δ_{col} [m]	T_{eff} [sec]	$C_{s,max}$ [g]	μ_{RCy}	μ_{col}	R_{RCy}	R_{col}	$S_{ae,RCy}$ [g]	$S_{ae,col}$ [g]	PGA_{RCy} [g]	PGA_{col} [g]	Storey involved in collapse	$\Delta_{PGA,NC}$ [%]	$\Delta_{PGA,RCy}$ [%]
1	-	-	1.35	0.08	0.56	2.10	0.09	0.61	4.34	0.71	4.34	0.06	0.40	0.16	0.66	1+2+3+4	-	-
2	f_c	$\mu-1.7\sigma$	1.57	0.09	0.45	2.36	0.09	0.57	2.84	0.64	2.84	0.06	0.25	0.16	0.51	1+2+3+4	-23.3	2.5
3	"	$\mu+1.7\sigma$	1.19	0.07	0.67	1.85	0.09	0.70	6.45	0.79	6.45	0.07	0.60	0.16	0.83	1+2+3+4	25.5	1.5
4	f_y	$\mu-1.7\sigma$	1.32	0.07	0.57	2.02	0.08	0.64	5.13	0.74	5.13	0.06	0.43	0.15	0.69	1+2+3+4	3.9	-6.5
5	"	$\mu+1.7\sigma$	1.38	0.09	0.55	2.14	0.10	0.61	3.75	0.69	3.75	0.07	0.37	0.17	0.64	1+2+3+4	-3.0	7.4
6	θ_y	$\mu-1.7\sigma$	1.34	0.08	0.56	2.07	0.09	0.62	4.44	0.71	4.44	0.06	0.40	0.16	0.67	1+2+3+4	0.9	-0.8
7	"	$\mu+1.7\sigma$	1.36	0.08	0.56	2.12	0.09	0.62	4.22	0.71	4.22	0.06	0.38	0.16	0.65	1+2+3+4	-1.2	1.5
8	θ_u	$\mu-1.7\sigma$	1.35	0.08	0.26	2.10	0.09	0.61	2.01	0.71	2.01	0.06	0.18	0.16	0.37	1+2+3+4	-44.3	0.0
9	"	$\mu+1.7\sigma$	1.35	0.08	1.14	2.10	0.09	0.61	8.80	0.71	8.80	0.06	0.80	0.16	1.13	1+2+3+4	71.1	0.0

does not change significantly and the beneficial effect of the higher strength in X direction compared with Y direction, due to the orientation of column elements – which, following the parallel plane frame configuration, provide a higher strength in X direction – leads to a higher $S_{ae,collapse}$ and also to a higher $PGA_{collapse}$ in X direction. The difference between the values of $S_{ae,collapse}$ in X and Y directions is higher than the difference between the values of $PGA_{collapse}$, due to the difference in T_{eff} between the two directions.

As far as first RC yielding is concerned, column orientation leads to a lower displacement capacity in X direction compared with Y direction; nevertheless, beneficial effect of the higher strength in X direction leads to a higher seismic capacity in terms of S_{ae} also at this Limit State. Again, due to the difference in T_{eff} between the two directions, the difference between the values of $S_{ae,RCy}$ in X and Y directions is higher than the difference between the values of PGA_{RCy} .

Bare Frame – Comparison Between X and Y Directions

If a comparison is carried out between the seismic capacity in X and Y directions for Model #1 see Fig. (21a and 21b) a clear difference is observed in the lateral response of the building: the column-sway storey mechanism in X direc-

tion leads to a higher strength but also to a lower ductility, whereas the opposite happens in Y direction, where a global mechanism occurs. Globally, $S_{ae,collapse}$ in Y direction is lower than in X direction, since – due to the higher deformability (higher value of S_{dy}) – in Y direction the ductility capacity $\mu_{collapse}$ is not as higher (compared with the X direction) as the displacement capacity $\Delta_{collapse}$. Hence, the lower strength is not effectively counterbalanced by the higher ductility capacity. Nevertheless, due to the difference in T_{eff} between the two directions, in terms of PGA a higher capacity is observed in Y direction.

As far as first RC yielding is concerned, a similar displacement capacity is observed, but the higher value of S_{dy} and, above all, the lower strength, lead to a lower $S_{ae,RCy}$ in Y direction. Due to the difference in T_{eff} , this difference decreases in terms of PGA, but, however, a higher PGA_{RCy} is observed in X direction.

X Direction – Comparison Between Uniformly Infilled, Pilotis and Bare Frames

A first comparison can be carried out between the IN2 curves in terms of $S_{ae}(T_{eff})$ for Uniformly infilled, Pilotis and Bare frame. Actually, the MDOF displacement capacity at

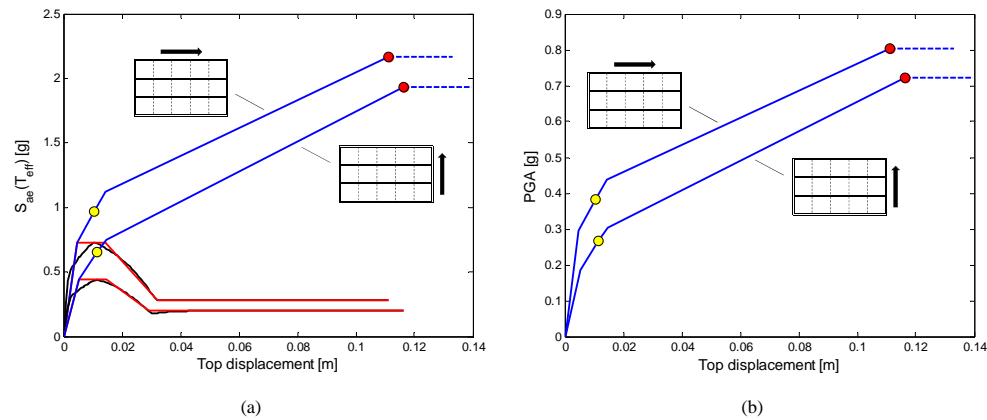


Fig. (19). Obtained (black) and idealized (red) pushover curves and IN2 (blue) curves in terms of $S_{ae}(T_{eff})$ (a) and PGA (b) for Model #1 in X and Y directions (Uniformly Infilled frame).

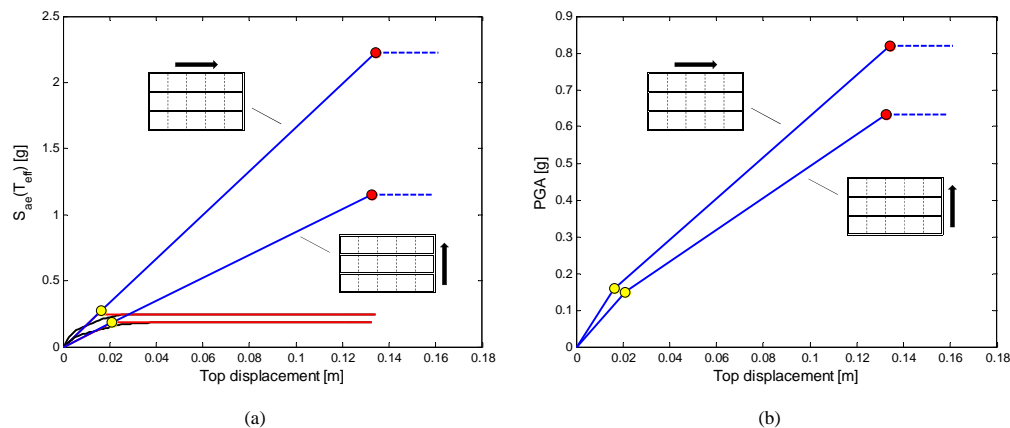


Fig. (20). Obtained (black) and idealized (red) pushover curves and IN2 (blue) curves in terms of $S_{ae}(T_{eff})$ (a) and PGA (b) for Model #1 in X and Y directions (Pilotis frame).

Collapse is almost coincident between the Uniformly infilled and the Pilotis frame since the collapse mechanism involve in both cases RC columns at the 1st storey; nevertheless, a lower displacement capacity of the equivalent SDOF is observed in Fig. (22a), due to the lower modal participation factor Γ for the Pilotis frame. The displacement capacity of the Bare frame is the highest one since in this frame the collapse mechanism involves the columns at the 3rd storey, which are characterized by a lower axial load and, hence, by a higher ductility. The high base shear capacity of the Uniformly infilled frame leads to much higher values of S_{ae} for relatively low displacement values (e.g., DL Limit State). As the ductility increases, the detrimental effect given by the increase in the slope of the IN2 curve, due to the drop of strength represented through the parameter r_u , prevails over the higher base shear and, at NC Limit State, the S_{ae} capacity of the Uniformly infilled frame is lower compared with the corresponding capacity of the Pilotis frame. The higher displacement capacity of the Bare frame is counterbalanced by its lower strength but, above all, by the lower stiffness of the pushover curve, resulting in lower S_{ae} capacity at both Limit States, compared with other frames.

Nevertheless, in order to compare the seismic capacity of different frames, it is more correct to plot the IN2 curves in terms of PGA since this can be considered as a *common*

measure of seismic intensity, whereas it is not appropriate to compare the seismic capacity in terms of spectral acceleration if the periods of the various frames are significantly different. It can be observed that the relative ratios between the seismic capacities of the Uniformly infilled and the Pilotis frame do not change significantly if PGA is considered instead of $S_{ae}(T_{eff})$ see Fig. (22b), because in both cases the period T_{eff} is lower than T_C , hence the spectral acceleration is on the constant branch of the spectrum. Therefore, when PGA is considered instead of $S_{ae}(T_{eff})$, the only (slight) change in the relative ratios between the seismic capacities of the frames is due to the little change in F_0 with the different PGA. A severe change, instead, is observed if the seismic capacity of the Bare frame is considered in terms of PGA. In this case, due to the relatively high period T_{eff} , the evaluation of the seismic capacity significantly changes: the PGA capacity at NC Limit States remains the lowest, compared with Uniformly infilled and Bare frames, but anyhow the relative distance between the seismic capacity of the Bare frame and of remaining frames significantly decreases. Moreover, at DL Limit State the PGA capacity of the Bare frame is higher, compared with the capacity of the Pilotis frame.

In conclusion, the better seismic performance at DL Limit State is shown by the Uniformly infilled frame, due to the high contribution of infill elements in terms of stiffness

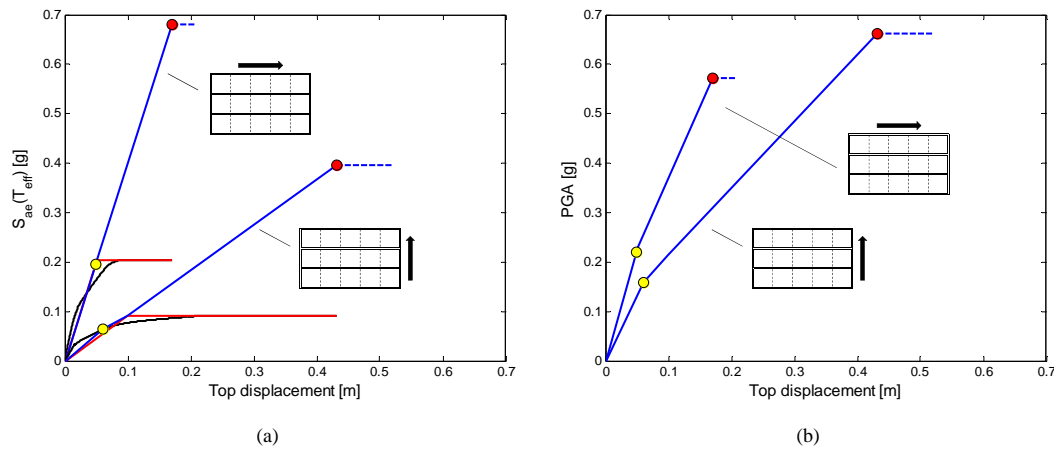


Fig. (21). Obtained (black) and idealized (red) pushover curves and IN2 (blue) curves in terms of $S_{ae}(T_{eff})$ (a) and PGA (b) for Model #1 in X and Y directions (Bare frame).

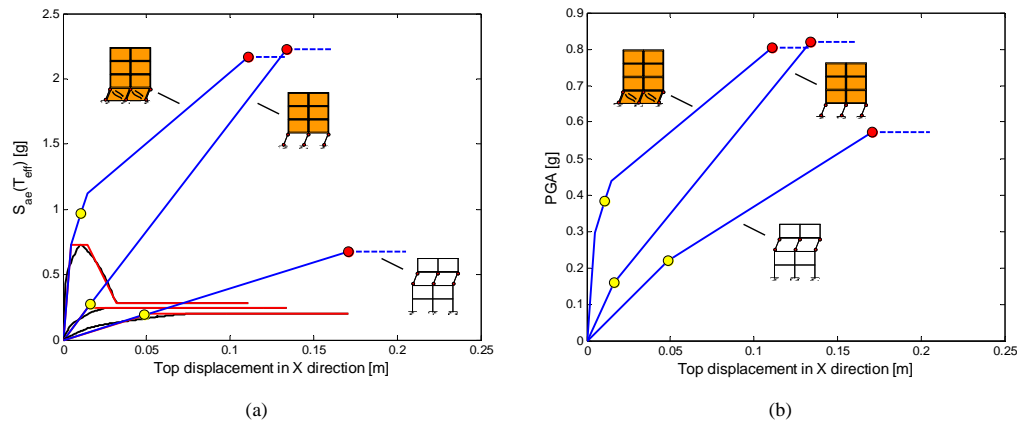


Fig. (22). Obtained (black) and idealized (red) pushover curves and IN2 (blue) curves in terms of $S_{ae}(T_{eff})$ (a) and PGA (b) for Model #1 in X direction (Uniformly infilled, Pilotis and Bare frames); collapse mechanism (c.m.) is reported.

and strength, whereas at NC Limit State the better performance is shown by the Pilotis frame since for higher values of ductility the detrimental effect due to the brittle behavior of infills counterbalances their strength contribution. At NC Limit State the Bare frame shows the worst behavior. Nevertheless, it is to be noted that in this frame the collapse mechanism, even without infills, is an unfavorable soft-storey mechanism, as may happen in existing buildings not designed according to modern principles such as weak beam/strong column condition.

Y Direction – Comparison Between Uniformly Infilled, Pilotis and Bare Frames

In Y direction, different considerations can be made if the seismic capacity of Uniformly infilled and Pilotis frames are compared, see Fig. (23a). In the former case, the soft-storey collapse mechanism takes place at the 2nd storey, where RC columns are characterized by a slightly higher deformation capacity compared with the 1st storey, where the soft-storey collapse mechanism takes place in the Pilotis frame. Hence, the detrimental effect (in terms of SDOF displacement capacity) of higher modal participation factor for the Uniformly infilled frame is partially counterbalanced.

Moreover, in this direction the higher strength provided by the Uniformly infilled frame counterbalances the detrimental effect given by the increase in the slope of the IN2 curve, due to the drop of strength, thus leading to a higher capacity, in terms of $S_{ae}(T_{eff})$, at both Limit States. If the Bare frame is considered, it is noted how the global collapse mechanism provides a significantly higher ductility, compared with other frames, but also a significantly lower strength, globally resulting in a lower capacity, in terms of $S_{ae}(T_{eff})$, at both Limit States.

If the seismic capacity is assessed in terms of PGA, see Fig. (23b), different effects are observed: first, the relative distance between seismic capacity of Uniformly infilled and Pilotis frames decreases, especially at NC Limit State, mainly because in this case T_{eff} is lower than T_C for the Uniformly infilled frame and higher than T_C for the Pilotis. Therefore, when PGA is considered instead of S_{ae} , seismic capacities of these frames become much closer to each other. For the same reason, the assessment of the seismic capacities of the Bare frame, which is characterized by a much higher value of T_{eff} , significantly changes if PGA is considered instead of $S_{ae}(T_{eff})$, showing a seismic capacity higher than the Pilotis frame at both Limit States.

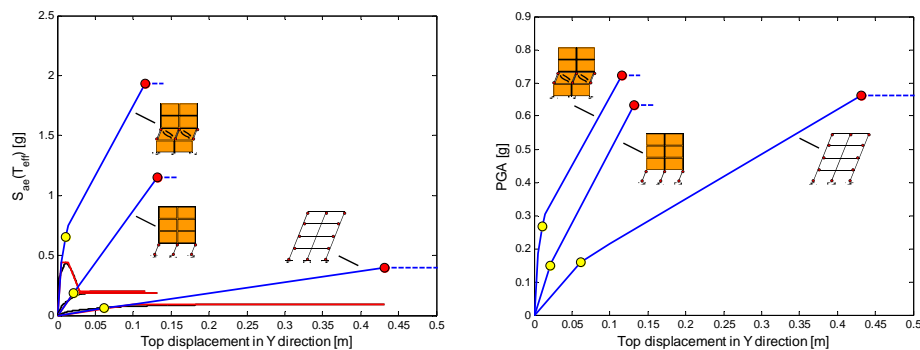


Fig. (23). Obtained (black) and idealized (red) pushover curves and IN2 (blue) curves in terms of $S_{ae}(T_{eff})$ (a) and PGA (b) for Model #1 in Y direction (Uniformly infilled, Pilotis and Bare frames); collapse mechanism (c.m.) is reported.

It is observed how, in Y direction, opposite to X direction, the seismic performance of the Bare frame at NC Limit State is better compared with the Pilotis frame and very closer to the Uniformly infilled frame. This is a beneficial consequence of the higher ductility provided by the global collapse mechanism observed in the Bare frame in this direction. At DL Limit State, the beneficial influence of the higher stiffness and strength provided by infills leads to a higher PGA for this kind of frame.

CONCLUSIONS

In this paper, the influence of main parameters influencing the seismic capacity of the case study structure has been investigated through a sensitivity analysis. Such analysis has shown that the rotational capacity of columns, directly influencing the displacement capacity, has the highest influence on the PGA capacity at NC for all of the investigated frames. Concrete compressive strength significantly influences the capacity at collapse, too, through its influence on the rotational capacity of columns. As far as PGA capacity at DL is considered, mechanical characteristics of infills have the highest influence on the response of the Uniformly Infilled frame, which is assumed to be at DL when the last infill in a storey reaches its maximum resistance, whereas for Pilotis and Bare frames the steel yield strength has the highest (but relatively lower) influence on PGA capacity at DL, which is assumed to correspond to the first yielding in RC members. Presence of infills, as expected, significantly influences the collapse mechanism: the Pilotis frame collapses under a soft-storey mechanism at the 1st storey, where infills are not present, in all of the cases investigated in the sensitivity analysis, whereas a soft-storey mechanism is always observed in the Uniformly Infilled frame, too, but the storey involved by the collapse mechanism can change, mainly depending on mechanical characteristics of infills.

Numerical results confirm how the presence of infills provides a beneficial increase in stiffness and strength which may or may not be counterbalanced by the detrimental effect due to the sudden loss of strength, which leads to an increase in displacement demand when a certain threshold of seismic intensity is exceeded. The latter effect is expressed in the adopted R- μ -T relationship [14] by the coefficient r_u , and is reflected in the decreasing slope of IN2 curves with r_u decreasing.

Compared with the Bare frame, in the Uniformly infilled frame the beneficial influence of the increase in stiffness and strength prevails over the detrimental influence of the brittle behavior, both in X and Y direction. The detrimental effect of an irregular distribution of infills – leading, as expected, to a localization of inelastic displacement demand at the bottom bare storey – is evident in Y direction, where the displacement capacity of the Bare frame is significantly higher, due to the formation of a favorable global collapse mechanism. On the contrary, in X direction, where the formation of an unfavorable column-sway storey mechanism is observed also in the Bare frame, the detrimental effect of an irregular distribution of infills is not observed. These observations confirm how the evaluation of the infill influence on seismic behavior cannot be independent of the evaluation of the seismic behavior shown by the bare structure.

Moreover, it should be pointed out that special attention should be addressed to the potential brittle failure mechanisms due to the local interaction between infills and structural elements – which have not been accounted for herein – especially for existing RC buildings that have not been designed adopting general principles and detailing rules prescribed by modern seismic codes according to Capacity Design philosophy.

CONFLICT OF INTEREST

The authors confirm that this article content has no conflicts of interest.

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